International Journal of Psychosocial Rehabilitation, Vol. 23, Issue 04, 2019
ISSN: 1475-7192

ENHANCING STRENGTH: FIBER-REINFORCED POLYMER COMPOSITES IN PLAIN CEMENT CONCRETE BEAMS

[1]Thirunagiri Sruthi, [2]Eslavath Sudhakar, [3]Lakavath Krishna Prasad, [4]Kakati Sri Sai Jyothi

ABSTRACT

There is now a lot of research being done on the possible applications of fiber-reinforced polymer wraps, covers, and sheets for the preservation and protection of metropolitan populations. It has been demonstrated that treating ageing structures using fiber-reinforced polymer (FRP) is a great method of strengthening and repairing them. FRP repair solutions offer a more affordable option than traditional repair techniques and materials. Regular beams' flexural and shear behaviour have been studied using glass fibre reinforced polymer (GFRP) sheets. GFRP sheets reinforced with epoxy were used in a balanced two-point manner to test cement footings built remotely until they failed. Three sets of samples were prepared for this RCT (randomised controlled experiment). In SET I, two flexure cubes were cast and reinforced with continuous sheets of glass fibre reinforced polymer (GFRP), while one weak cube served as a control. SET II controlled the behaviour of the third shear-weak cylinder and improved the shear characteristics of two of the cylinders using continuous glass fibre reinforced polymer (GFRP) sheets. In SET III, three flexure-weak prisms were cast, two of which were strengthened with glass fibre reinforced polymer (GFRP) sheets and one of which functioned as the controlled beam. To reinforce or repair the samples, different configurations and amounts of GFRP sheets and resins are utilised. Furthermore, tensile strength is tested following the casting of twelve cylinders, three of which are retrofitted with glass fibre reinforced polymer (GFRP) sheets, and one of which is used as a control.

In a controlled laboratory context, the load, deflection, and possible failure modes of each beam were determined. There is also a thorough discussion of the creation and application of GFRP sheets for reinforcing RC beams. The relationship between beams' ultimate load-bearing capability and failure mechanism and the amount of GFRP layers employed in their manufacture is examined.

There is now a lot of research being done on the possible applications of fiber-reinforced polymer wraps, covers, and sheets for the preservation and protection of metropolitan populations. It has been demonstrated that treating ageing structures using fiber-reinforced polymer (FRP) is a great method of strengthening and repairing them. FRP repair solutions offer a more affordable option than traditional repair techniques and materials.

Regular beams' flexural and shear behaviour have been studied using glass fibre reinforced polymer (GFRP) sheets. GFRP sheets reinforced with epoxy were used in a balanced two-point manner to test cement footings built remotely until they failed. Three sets of samples were prepared for this RCT (randomised controlled experiment). In SET I, two flexure cubes were cast and reinforced with continuous sheets of glass fibre reinforced polymer (GFRP), while one weak cube served as a control. SET II controlled the behaviour of the third shear-weak cylinder and improved the shear characteristics of two of the cylinders using continuous glass fibre reinforced polymer (GFRP) sheets. In SET III, three flexure-weak prisms were cast, two of which were strengthened with glass fibre reinforced polymer (GFRP) sheets and one of which functioned as the controlled beam. To reinforce or repair the samples, different configurations and amounts of GFRP sheets and resins are utilised. Furthermore, tensile strength is tested following the casting of twelve cylinders, three of which are retrofitted with glass fibre reinforced polymer (GFRP) sheets, and one of which is used as a control.

In a controlled laboratory context, the load, deflection, and possible failure modes of each beam were determined. There is also a thorough discussion of the creation and application of GFRP sheets for reinforcing RC beams. The

[123]Assistant Professor, Department of Civil Engineering, Abdul Kalam Institute of Technological Sciences, Kothagudem, Telangana

[4]Student, Department of Civil Engineering, Abdul Kalam Institute of Technological Sciences, Kothagudem, Telangana

impact of multiple GFRP layers on the ultimate load-bearing capacity and failure mechanism of the beams is discussed.

I. INTRODUCTION

1.1 GENERAL

The most important problems in structural design applications are the maintenance, repair, and redesigning of structural components. Furthermore, numerous structures constructed in the past in different parts of the globe using more established design standards are structurally unsafe under the new regulations. Because replacing such insufficient components of structures costs a significant amount of public money and time, strengthening has emerged as a viable way of increasing their load bearing capacity and longevity. The study of a few techniques for repairing or strengthening infrastructure owing to untimely deterioration of buildings and constructions has led to the study of a few strategies for fixing or strengthening infrastructure. One of the challenges in strengthening significant designs is deciding on a strengthening method that would enhance the structure's strength and workability while also taking into account limitations such as constructability, building activities, and budget. Due to a variety of reasons, primary strengthening may be required.

- Additional strength may be needed to put greater loads on the structure. This usually occurs when the structure's usefulness changes and a greater load-carrying capacity is required. When a structure receives more mechanical equipment, filing, planters, or other things, the structure requires greater strength.
- Strengthening, retrofitting, or repairing the structure may be needed to allow it to withstand loads that were not anticipated during the initial design process.
- This may occur when initial strengthening is required for loads resulting from wind and seismic forces, or when additional development of impact load protection is required.

Because of an insufficiency in the structure's capability to the loads discussed previously, more strength may be needed. The insufficiency may be due to deterioration (e.g., reinforcing corrosion and loss of a significant section), structural damage (e.g., vehicle collision, severe wear and tear, high loading, and fire), or errors in the original design process or construction execution (e.g., lost or missing reinforcement and deficient concrete strength).

When dealing with such circumstances, each project has its unique set of constraints and demands. Whether dealing with space constraints, constructability constraints, solidity requirements, or a variety of other problems, each project requires a great deal of creativity in coming up with a strengthening solution.

The majority of primary strengthening focuses on the structural component's ability to withstand at least one of the following interior forces caused by loading: flexure, shear, axial, and twisting. Strengthening may be fine-tuned by lowering the amount of these pressures or increasing the member's resistance to them. When dealing with such conditions, common strengthening methods, such as section expansion, have their own set of limitations and expectations. Whether dealing with space constraints, constructability constraints, durability needs, or any other problem, each project requires a tremendous degree of ingenuity in order to come up with a strong solution.

The majority of structural strengthening methods include increasing the structural element's capacity to withstand one of the following internal loading forces: flexure, shear, axial, or torsion. Strengthening is achieved by either lowering the amount of these pressures or increasing the member's resistance to them. Strengthening techniques such as section expansion, externally bonded reinforcement, post-tensioning, and supplementary supports may all be used to enhance the strength and serviceability of a structure.

Passive strengthening techniques are used when extra loads beyond the design loads are encountered during installation. Bonding steel plates or fiber-reinforced polymer (FRP) composites to structural elements are examples of passive strengthening techniques.

Active strengthening systems usually engage the structure instantly, and they work by applying external pressures to the member to counterbalance the effects of internal stresses. External post-tensioning systems or jacking the member to alleviate or transfer existing load are examples of this. The primary difficulty is to create composite behaviour between the old structure and the additional strengthening components, whether passive or active.

The best method for reinforcing or strengthening a structure requires careful consideration of a number of factors, including the following technical issues:

- Increase in strength magnitude.
- The impact of relative members' stiffness changes.
- The scope of the project (use of special materials and methods might be of less cost are effective on small projects).

- The existing concrete's bond. Integrity of the substrate.
- Concerns about the environment

1.2 STRENGTHENING USING FRP COMPOSITES

A few years ago, the construction industry started to use FRP for structural reinforcement, mainly in combination with other building materials such as wood, steel, and cement. FRPs have a few more developed characteristics, such as a high strength-to-weight ratio, a high firmness-to-weight ratio, plan flexibility, non-destructiveness, high weakness strength, and ease of usage. Only a few experts have focused on the use of FRP sheets or plates connected to cement footers. Fortification using glue-fortified fiber-supported polymers has shown to be a viable approach for a variety of large-scale designs such as sections, shafts, pieces, and dividers. FRP materials are increasingly being used for exterior support of current large designs since they are non-destructive, non-attractive, and resistant to various synthetic chemicals. The use of remotely fortified glass fiber-supported polymers (GFRP) to enhance the flexural, shear, and torsional limits of RC radiates has been shown in previous research. The adaptable glass fibre sheets have been revealed to be profoundly appealing for reinforcing RC radiates because to their adaptable nature and ease of care and application, along with high elasticity weight proportion and solidity.

In the last several years, the use of fibre supported polymers (FRPs) for the recovery of existing significant designs has grown rapidly. FRP has been proven to be effective in strengthening cement footers that are prone to flexure, shear, and twist. Regrettably, present Indian significant plan standards (IS Codes) prohibit any preparations for flexural, shear, or torsional strengthening of underneath people using FRP materials. Due to the lack of plan principles, groups were formed between the exploration community and industry to study and improve the use of FRP in the flexural, shear, and torsional repair of existing structures. FRP is a composite material made up mostly of high-strength carbon, aramid, or glass filaments embedded in a polymeric grid (e.g., thermosetting sap), with the strands serving as the primary load-carrying component.

This support may take the shape of prefabricated overlays or adjustable sheets, among other options. The coverings are pre-relieved tough plates or shells that are inserted by glueing them to the significant surface using thermosetting pitch. The sheets are either dry or pre-impregnated with tar (known as pre-preg) before being installed on a large surface and repaired. Wet lay-up is the term for this type of installation. High rigidity, lightweight, great solidity, high weariness strength, and amazing sturdiness are only a few of the physical and mechanical characteristics offered by FRP materials. FRP frameworks are easy to implement due to their lightweight and formability. These frameworks are a fantastic option for exterior support since they are non-destructive, non-attractive, and mostly resistant to synthetic chemicals. When compared to traditional reinforcing methods, the characteristics of FRP composites and their flexibility have resulted in significant cost savings and reduced office shutdown time (e.g., area broadening, outer post-tensioning, and fortified steel plates).

Reinforcing using remotely reinforced FRP sheets has been shown to be useful for a variety of RC underlying components. To provide additional flexural strength, FRP sheets may be attached to the strain side of main individuals (e.g., chunks or pillars). To provide additional shear strength, they may be attached to the web sides of joists and radiates or folded over portions. They may be folded over portions to increase significant control and, as a result, segment strength and flexibility. FRP sheets may be used to strengthen cement and brickwork separators to make them more likely to resist parallel loads, as well as circular constructions (such as tanks and pipes) to resist internal pressure and reduce consumption. Starting today, a small number of huge square metres of surface reinforced FRP sheets have been used in a variety of fortification operations all over the globe.

Objectives

- The primary goal is to investigate the use of fibre reinforced polymer to improve the strength of current as well as old and worn out structures, in order to either accomplish or extend the structure's anticipated life.
- To compare the strength values of a structure that have been decreased by fire activity to those of a normal structure.
- To draw a curve depicting the impact of fire reaching various depths of structural components.
- Calculate the % improvement in strength due to the use of FRP.

II. PROPOSED METHODOLOGY

The goal of the project is to determine the compressive and flexural strength of cubes, as well as the deflection of reinforced concrete beams. In this concrete blend model, the extents of components that will produce cement with

the required appealing characteristics are chosen. The proportions of the M20 combination should be preferred in such a way that the resulting concrete is craved workability while new and can be set and compacted easily for the expected reason, the proportions of the M20 combination should be preferred in such a way that the resulting concrete is craved workability while new and can be set and compacted effortlessly for the expected reason, the proportions of the M20 combination should be preferred in such a way that the resulting concrete is craved workability while new and Consider the unique example control specimen and retrofitting specimen in ordinary Portland cement (OPC) with water, fine aggregate, coarse aggregate (20mm) and (12mm) and the beam. To improve the beam's quality, Fiber Reinforced Polymers (FRP) sheets or plates are utilised. Reinforced glass fibre fortified polymer (GFRP) guided test on 28 days to maximise the quality of retrofitting sample with unique resin characteristics such as epoxy resin, GP resin, and isophthalic resin. A total of twelve number beams were cast, with the specific test results being recorded. The beams were flexural strength tested after being upgraded with single layer GFRP utilising complete form wrapping. The flexural strength and deflection achieved by changing the load in various specimens were the characteristics to be considered in evaluating the execution of the beam.

III. MATERIALS

3.1 CONCRETE

Concrete is a building material made out of portland cement and water, as well as sand, gravel, crushed stone, or inert materials such expanded slag or vermiculite. Cement and water combine to create a mixture that hardens into a strong, stone-like mass via chemical reaction. Aggregates are the inert components, and for economy, just enough cement paste is required to cover all of the aggregate surfaces and fill all of the gaps. The concrete paste is flexible and may be moulded into any shape or troweled to create a smooth finish. Hardening starts quickly, although measures are taken, typically by covering, to prevent fast moisture loss, since water is required to complete the chemical reaction and enhance strength. However, too much water causes the concrete to become porous and fragile. The nature of the concrete is mainly determined by the quality of the paste produced by the cement and water. Designing the mixture refers to the proportioning of the components in concrete, and most structural work requires concrete with compressive strengths of 15 to 35 MPa. A rich combination for columns could be 1 volume of cement to 1 volume of sand and 3 volume of stone, whereas a lean mixture for foundations might be 1:3:6. Concrete may be constructed as a thick mass that resembles artificial rock, with chemicals added to make it waterproof, or it can be made porous and extremely permeable for filter bed applications. To generate minute bubbles for porosity or light weight, an air-entraining chemical may be applied. Concrete typically takes at least 7 days to fully harden. The hydration of the tricalcium aluminates and silicates causes the steady rise in strength. Originally, angular sand was recommended for use in concrete, but rounder grains are now preferable. The stone is typically shattered into sharp pieces. Concrete's weight varies depending on the kind and quantity of rock and sand used. The density of concrete including trap rock may be as high as 2,483 kg/m3. Steel bar, also known as rebar or mesh, is inserted in structural components to enhance the tensile and flexural strengths. Concrete is extensively utilised in precast units such as block, tile, sewage, and water pipe, as well as decorative goods, in addition to structural applications.

The study was carried out using Portland slag cement (PSC) of grade 43. Physical characteristics were examined in line with Indian Standard standards. Clean river sand, passing through a 4.75 mm sieve with a specific gravity of 2.68, was utilised as the fine aggregate in this study. According to Indian Standard standards, the fine aggregate grading zone was zone III. As coarse aggregate, machine crushed granite broken stone with an angular form was utilised. The coarse aggregate had a maximum size of 20 mm and a specific gravity of 2.73. Both mixing and curing of concrete were done using ordinary clean portable water that was devoid of suspended particles and chemical compounds.

The largest aggregate size utilised in concrete was 20 mm. To obtain a strength of 20 N/mm2, a nominal concrete mix of 1:1.5:3 by weight is utilised. The water cement ratio is set at 0.5. To measure the compressive strength of concrete, three cube specimens were cast and tested at the time of the beam test (at the age of 28 days). The concrete's average compressive strength was 31N/mm2.

3.1.1 Cement

Cement is a powdered substance that may be formed into a paste by adding water and then moulded or poured into a solid mass. Cements refer to a variety of organic compounds used for attaching or fastening objects, although they are classed as adhesives, while the word cement refers to a building material. Portland cement is the most commonly

utilised of the building cements. It's a bluish-gray powder produced by finely grinding clinker made by heating an intimate combination of calcareous and argillaceous minerals to high temperatures. A combination of high-calcium lime stone, sometimes known as cement rock, and clay or shale is used as the primary raw material. Some cements may also include blast-furnace slag, which is known as portland slag cement (PSC). Iron oxide is primarily responsible for the cement's colour. The colour would be white if there were no impurities, but neither the colour nor the specific gravity is a quality indicator. The study utilised Portland slag cement (PSC)–43 grade with a specific gravity of at least 1.0.

3.1.2. Fineaggregate

Fine aggregate, often known as sand, is a collection of mineral grains formed by the dissolution of rocks. It differs from gravel solely in terms of grain size or particle size, but not from clays that contain organic elements. Sands that have been sifted and separated from organic material by the action of water currents or winds over dry areas have typically consistent grain sizes. Typically, commercial sand is sourced from riverbeds or sand dunes that were created by wind activity. Sand makes up a large portion of the earth's surface, and the sands are typically quartz and other siliceous minerals. Silica sands, which are typically over 98 percent pure, are the most economically valuable. Beach sands are typically devoid of biological debris and contain smooth, spherical to ovaloid particles due to the abrasive impact of waves and tides. The white beach sands are mostly silica, but they may also include zircon, monazite, garnet, and other minerals, and they're utilised to extract different elements.
Sand is used in the production of mortar and concrete, as well as in polishing and sandblasting. In foundries, sands with a little amount of clay are used to make moulds. Filtering water is done using clear sands. The quality of the sands used for this purpose varies depending on where they are available. A silica sand used in concrete and cement testing is known as standard sand. In this experiment, fine aggregate collected from the Koel riverbed was utilised, which was free of any organic contaminants. The fine aggregate had a specific gravity of 2.68 after passing through a 4.75 mm screen. According to Indian Standard standards, the fine aggregate grading zone was zone III.

GLASS FIBER SHEET

These are fibres that are widely utilised in the naval and industrial sectors to create medium-high-performance composites. Their great strength is a distinguishing feature. Because glass fibres have a lower Young modulus of elasticity (70 GPa for E-glass) and have a poorer abrasion resistance than carbon or aramid fibres, they must be handled with extreme care. They also have a poor fatigue strength and are prone to creep. Fibers are subjected to sizing procedures that serve as coupling agents to improve the connection between fibres and matrix, as well as to protect the fibres from alkaline chemicals and moisture. These treatments help to improve the composite material's durability and fatigue performance (both static and dynamic). FRP composites based on fibreglass are often referred to as GFRP composites centred on carbon fibre are commonly referred to as GFRP composites, as shown in Figure 1. The resin is one of the most fundamental components that influences the composites' execution.

Fig.3.1.E-Glassfiber sheets

The thermoplastics and thermo sets are the two types of resins. At room temperature, a thermoplastic resin maintains its strength. When heated, it liquefies and hardens when cooled. Long-chain polymers do not cross-link artificially. The thermo set resin composites with this trademark are particularly appealing for structural applications. Unsaturated polyesters, epoxies, and vinyl esters are the most often recognised tars used in composites, while polyurethanes and phenolics are the least common.

3.1.4 Epoxies

Glycidyl ethers and amines are the most common types of epoxies used in composites. Material characteristics

and cure rates may be tailored to suit the requirements of the job. Epoxies are mostly used in the marine, automotive, electrical, and apparatus industries. Epoxy resins' high density restricts its use to particular methods, such as shaping, fibre slowing, and hand lay-up. The right curing experts should be carefully selected since they will affect the kind of compound reaction, pot life, and final material characteristics. Despite the fact that epoxies may be expensive, they may be justified when better performance is required. The properties of unique polymeric resins, such as epoxy resin, general purpose (GP), and isophthalic resin, are listed in the table below.

TABLE:-1

Properties of Resin	Epoxy	GP	ISO
Glass transition temperature	120-130	80	80
Tensile strength	85N/mm2	55 N/mm2	55N/mm2
Tensile Modulus	10500 N/mm2	3450 N/mm2	3300 N/mm2
Elongation at break	0.8%	2.2 %	2.5-3.55 %
Flexural strength	112 N/mm2	80 N/mm2	125 N/mm2
Flexural Modulus	10000 N/mm2	3450 N/mm2	3400 N/mm2
Compressive	190 N/mm2		

IV. METHODLOGY

This chapter focuses on improving an analytical model for analysing and designing flexural beams reinforced with externally bonded glass fibre reinforced polymer composite sheets. The goal of this study is to correctly anticipate the flexural behaviour of reinforced concrete beams reinforced with glass fibre reinforced plastic sheet. Before the major experiment, several essential experiments on cement and fine aggregate are carried out to get preliminary project information.

4.1 Tests on Materials

The following are the many tests performed on the materials used in the project; various apparatus and equipment are used to test the quality and characteristics of the materials. These tests are carried out to determine the quality of the materials, and the findings aid in the mix design calculations. These fundamental tests must be carried out in order to get effective findings.

1. Gravity Specification
2. Relative Consistency
3. Analysis of Sieves
4. Cement Fineness
5. Absorption of water
6. Cement setting (first and final)

4.2. Specific Gravity

The ratio of a material's density to the density of a reference substance is known as specific gravity.

a) Specific Gravity of Cement

Specific gravity is defined as the ratio of a material's density to that of a reference substance, or the ratio of a substance's mass to that of a reference substance for the same volume, using water as the reference substance.

During the mix design calculation procedure, the material's specific gravity is utilised. For this test, a specific gravity bottle, a weight balance, kerosene, and cement were utilised as the equipment and supplies. Because kerosene does not react with cement, it is used instead of water.

The following procedures are used to determine the cement's specific gravity:

1. W1 g is the weight of an empty specific gravity bottle.
2. A third of the capacity of a specific gravity container is filled with cement, and the weight of the bottle is recorded as W2 gms.
3. Fill the remaining 2/3 of the capacity of the specific gravity bottle with kerosene, and record the weight of the bottle as W3 gms.
4. Now carefully clean the specific gravity container and fill it solely with kerosene, noting the weight

W4 gms.

5. Now solely use pure water to fill the specific gravity container, and record the weight W5 gms.

b) Sieve Analysis

The particle size distribution of coarse and fine aggregates may be determined via sieve analysis. The aggregates are sieved according to IS: 2386 (Part I) – 1963. We do this by passing aggregates through various sieves that have been standardised by the IS code, and then collecting different sized particles left behind from different sieves. The mechanical sieve shaker is used to conduct the sieve analysis in the laboratory. The primary goal of the test is to determine which zone the aggregate we're utilising belongs to. Taking a 1000g sample and analysing it using a sieve. The findings are shown in the tables below. The experiments were carried out on both fine and coarse material. Each aggregate's fineness modulus is computed, and the results are shown below the table. Before putting aggregates through sieves, make sure they're dry and devoid of any organic components. The existence of lumps in the aggregate should be eliminated by pressing with your fingertips. The following formula should be used to compute and report the results:

1. Retained sample weight
2. 2% of the original weight was retained.
3. Percentage of total weight retained

TABLE :-2 Sieve analysis

Sieve size	Weight of aggregate (mm)	Weight Retained	% of weight Retained	Cumulative % Retained	Cumulative % of passing
4.75 mm	1000	25	2.5	2.5	97.5
2mm	1000	142	14.2	16.7	83.3
1mm	1000	187	18.7	35.4	64.6
600 µ	1000	247	24.7	60.1	39.9
300 µ	1000	277	27.7	87.8	12.2
150 µ	1000	91	9.1	96.9	3.1
75 µ	1000	29	2.9	99.8	0.2
pan	1000	2	0.2	100	0

4.3 Mix Design

Based on the quality of the materials used and their moisture content, concrete mix design recommends quantities of cement, fine aggregate, coarse aggregate, and water. The final mix proportions are recommended based on laboratory tests and mix design revisions. We can offer concrete with strength ranging from M10 to M100 and workability ranging from no slump to 150 mm slump value using mix design.

Some admixtures are also needed to improve concrete characteristics such as setting time, workability, and so on. In order to make the most use of these admixtures, they must be taken into account during the mix design calculations. Because an excessive amount of them may alter the characteristics of concrete and compromise its strength.

4.4 Casting of Moulds

This is where the project's experimentation begins. Everything we've done in the mix design calculation thus far has been theoretical. After calculating the mix ratio, the cubes are cast at this step. The casting in this experiment is done by hand mixing. The procedures to take while mixing concrete are as follows.

1. Gather or obtain all of the materials and tools needed to complete the task.
2. Materials must be gathered and weighed correctly using a weighing scale according to the mix design.
3. The coarse aggregate is first spread out on the ground.
4. On top of the coarse material, fine aggregate and coir fibre are placed.
5. The cement is then added, and everything is mixed together.
6. The necessary amount of water is then gently added to achieve a homogeneous mixture.
7. The insides of the moulds and slump cone equipment are then thoroughly lubricated.
8. The concrete is then put in three levels in the slump cone and compressed 25 times with a tamping rod at each layer.
9. Once you've obtained the slump.

10. Finally, the concrete is poured into the moulds and compressed using a tamping rod.
11. Once the mixture has been placed in the moulds, the top surface of the mould is levelled and a smooth finish is applied with a trowel.
12. After that, the moulds are set aside for 24 hours.

After 24 hours, the concrete cubes are demolded.
They're then put in the curing tank to cure

Fig-4.1 Preparation of samples

4.5 Compressive strength test

The compressive strength of concrete is one of the most important tests on concrete; it allows us to assess the strength of the material. With the assistance of this test, we can evaluate whether or not the concreting has been done correctly. Depending on the size of the aggregates used, cubes of 150mm x 150mm x 150mm or 100mm x 100mm x 100mm are available for cube testing.
This concrete is poured into the mould and carefully manipulated to remove any voids. They are demoulded and placed in water to cure after a 24-hour period. This specimen's top surface should be levelled and smoothed. This is accomplished by applying cement paste to the whole surface of the specimen and spreading it evenly. After 7 days and 28 days of curing, these specimens are examined using a compression testing equipment. The specimen should be loaded at a rate of 140 kg/cm2 per minute until it fails. The Compressive strength of concrete is calculated by dividing the load at the specimen's failure by the specimen's area. Concrete cubes' compressive strength is tested according to the methods outlined in IS:516 -1959.
The following steps are used to determine compression strength:

1. Specimens kept in water must be examined as soon as possible while they are still wet.
2. Remove any remaining particles from the top surface. If the specimens are dry, they must be maintained in water for 24 hours before being tested.
3. Before the specimen is tested, the weight and measurements of the specimen are recorded with an inaccuracy of 0.2mm.
4. Before inserting the specimen into the machine for testing. Any loose sand or other debris from the surfaces of the specimens that will be in contact with the compression platens must be removed from the bearing surfaces of the testing equipment.
5. In the case of cubes, the specimen should be positioned in the machine such that the weight is given to the sides of the cubes as cast, rather than the top and bottom.
6. The specimen's axis should align with the spherically seated platen's centre of thrust.
7. As the spherically seated block is brought to bear on the specimen, gently rotate the moveable part by hand to produce a uniform section.
8. Gradually apply a load to the cube at a rate of about 140 Kg/cm2/min until the specimen breaks.
9. Make a note of the maximum load and any unique characteristics in the kind of failure.
10. Concrete cube average compressive strength=(load/area)

Fig-4.2. Specimen loaded into testing machine

V. RESULTS AND DISCUSSION

The load at which the flexural fracture first developed and the ultimate load at which the specimen fails are both recorded during the flexural strength test of control beams. These beams are classified as part of the SET I of beams. In SET II, three beams will be preloaded until flexural cracking occurs, and then retrofitted with GFRP sheets made from ISO resin. This resin is applied to the beams with a brush and will cure in 10 seconds, therefore the sheet should be wrapped around the beam as quickly as possible before the resin dries. Additionally, pressure should be given to the glass fibre layer in order for it to securely adhere to the beams' surface. The modified beams should be evaluated in a flexural strength testing machine once they have been properly placed. The ultimate load and deflection of the failure beam are meticulously recorded. In SET II, three beams will be preloaded until they develop a flexural fracture, then retrofitted with GFRP sheets and Epoxy resin. This resin is applied to the beams with a brush and will cure in 10 seconds, therefore the sheet should be wrapped around the beam as quickly as possible before the resin dries. Additionally, pressure should be given to the glass fibre layer in order for it to securely adhere to the beams' surface. The modified beams should be evaluated in a flexural strength testing machine once they have been properly placed. When recording the ultimate load and deflection data, extreme caution is required.

In SET III, three beams will be preloaded until they develop a flexural fracture, then retrofitted with GFRP sheets and Epoxy resin. This resin is applied to the beams with a brush and will cure in 10 seconds, therefore the sheet should be wrapped around the beam as quickly as possible before the resin dries. Additionally, pressure should be given to the glass fibre layer in order for it to securely adhere to the beams' surface. The modified beams should be evaluated in a flexural strength testing machine once they have been properly placed. When recording the ultimate load and deflection data, extreme caution is required.

Three beams will be pressured till flexural fracture and then retrofitted with GFRP sheets using GP resin in SET IV. This resin is applied to the beams with a brush and will cure in 10 seconds, therefore the sheet should be wrapped around the beam as quickly as possible before the resin dries. Additionally, pressure should be given to the glass fibre layer in order for it to securely adhere to the beams' surface. The modified beams should be evaluated in a flexural strength testing machine once they have been properly placed. When recording the ultimate load and deflection data, extreme caution is required.

Analysis of Compressive strength for different specimens

Thefollowingtables2and3illustratethecompressivestrengthofcementconcretecubeestimatedbytakingintoaccountheareaofthespecimenandbysuitablymodifyingtheload. The strengths of control specimens like (C1,C2,C3) and wrapped specimens like (W1,W2,andW3) are tested and assessed.

Table:-3CompressivestrengthforControlSpecimen

Sl.No	Specimens	Area of specimens	Weight of mould (Kg)	Load at failure (kN)	Compressive strength (N/mm²)	Average Compressive strength(N/mm²)
	C1	2500	8.49	550	24.44	24.48
	C2		8.62	539	23.9	
	C3		8.57	565	25.11	

Display the compressive strength of the control specimens in Table 2. In reality, the compression test of cubes is the most comprehensive examination of hardened concrete, partly because the bulk of concrete's famous distinguishing characteristics are qualitatively related to its compressive strength. Six test cubes (C1, C2, C3, C4, C5, and C6) must be cast for the compression test. The average of the three specimens' test strengths is used to calculate the sample's test strength (C1, C2, and C3). 3cubes also wraps with a variety of resins. 150mm concrete, 150mm concrete, 150mm concrete, 150mm concrete, 150mm concrete, 150mm concrete, 150 Concrete of the M20 grade is used to make the cubes. The area of the control specimen is set to 22500, and the weight of all control specimens is adjusted. The load observed to reduce down the compressive strength in CC1 when the load started is 560 and the strength is 24.48kN/mm2. The strength is found to take are treat in each control specimen where the weight is changed, and the divergences between the C1 strength and the C2 and C3 strength are 0.26 percent and 1.26 percent, respectively.

Table:-4 CompressivestrengthforwrappedSpecimen

LNo		Trials	Area of specimen (mm^2)	Load failure	Compressive strength	Average compressive strength N/mm^2
	W1(ISO)	1	2500	840	37.3	31.87
				860	31.2	
				610	27.11	
	W2(GP)	2		740	32.8	32.43
				760	33.7	
				695	30.8	
	W3(Epoxy)	3		710	31.55	29.47
				670	29.77	
				610	27.11	

Table 3 shows the compressive strength of wrapped specimens for which tests are carried out on different Glass fibre reinforced polymer specimens. W1 specimens are wrapped in isophthalic resin-bonded glass fibre reinforced polymer, while W2 and W3 specimens are wrapped with GPresin-bonded GFRP. When W cube is changed in each specimen, the load based on compressive strength increases.

Table:-5 Flexuralstrengthanddeflectionfordifferentspecimens

Test results	1	2	3	GP Resin 1	GP Resin 2	GP Resin 3
Initial crack load	3	2.5	1	1.5	5.2	3
Ultimate load(kN)	7	6	5	4	7	5
Deflection (mm)	.432	.417	.449	.573	.521	.545
Ultimate moment (kN-m)	.7	.2	.9	.83	.62	.9
Flexural strength (N/mm^2)	6.8	8.5	9.2	2.3	3.6	4

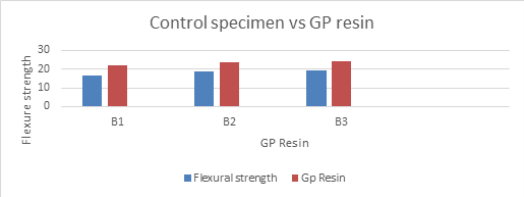
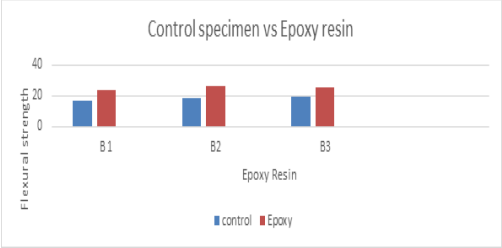


TABLE-6

Test results	B1	B2	B3	Epoxy Resin		
				R4	R5	R6
Initial crack load	33	32.5	31	40	33.5	30
Ultimate load(kN)	37	36	35	51	43	50
Deflection (mm)	0.432	0.417	0.449	0.562	0.583	0.625
Ultimate moment (kN-m)	2.7	3.2	2.9	4.15	4.32	4.18
Flexural strength (N/mm²)	16.8	18.5	19.2	23.5	26	25.6



VI. CONCLUSION

The following conclusions are derived from the findings of the test study of control and retrofitting beams of compressive strength, flexural strength, and deflection of cubes and prisms:

- The compressive strength of glass fibre reinforced with different resins joined cubes wrapped in GFRP sheets (C4, C5, and C6) with epoxy resin is 20.3 percent higher than control specimens (C1, C2, C3).
- The compressive strength of test cubes covered in GFRP sheets (C7, C8, and C9) bonded with ISO resin is 30.18 percent higher than the control specimen.
- The compressive strength of the specimens retrofitted with GP resin (C10, C11, and C12) is 32.4 percent higher than the control specimen.
- The following are the flexural strength test findings for control and retrofitted specimens.
- The average flexural strength of the control beamsB1,B2,B3 is 18.1 N/mm2.
- The flexural strength of the retrofitted specimens (R1,R2,R3), which are bonded with GFRP sheet and ISO resin, is 12.3 percent higher than the control specimen.
- GP resin-bonded specimens (R4,R5,R6) had a 28.2 percent higher strength than control specimens.
- Epoxy resin retrofitting test specimens (R7,R8,andR9) had 37.7% higher strength than control specimens.
- The control cylinders (CY1, CY 2, CY 3) have a tensile strength of 0.761 N/mm2 on average (after 7 days of curing) and 1.349 N/mm2 on average (after 7 days of curing). (for 28 days curing).
- Specimens bonded with GP resin (WY1,WY2,WY3) had 33 percent higher tensile strength after 7 days of curing and 37 percent more tensile strength after 28 days of curing than control specimens.
- Epoxy resin-bonded specimens (WY4, WY5, WY6) had 27 percent higher tensile strength than control specimens after 7 days and 46 percent higher tensile strength after 28 days.
- Specimens bonded with ISO resin (WY7, WY8, WY9) had a tensile strength of 12 percent higher than control specimens after 7 days of curing and 21.2 percent higher after 28 days of curing.
- When a beam is strengthened under shear, only flexural failure occurs, which provides adequate warning as opposed to brittle shear failure, which results in catastrophic beam collapse.

- The connection between the GFRP sheet and the concrete remains unbroken until the beam fails, indicating the composite action caused by the GFRP sheet.
- Using GFRP sheet to restore or upgrade the shear strength of beams may result in improved shear strength and stiffness with no visible shear fractures.
- The use of GFRP to restore the shear strength of beams is a very successful method.

REFERENCES

[1] T. Beitelman, R. Sowrirajan, M. A. Shahawy, M. Arockiasamy, M. Arockiasamy, M. Arockiasamy, M. Arockiasamy, M. Arockiasamy, M Composites:Part B 27B(1996)225-233 “Reinforced concreterectangularbeams strengthenedwithCFRPlaminates”

[2] Victor N. Kaliakin, Michael J. Chajes, and Ted F. Januszka Victor N. Kaliakin, Michael J. Chajes, and Ted F. Januszka “Analysisofexternallybondedwovencompositefabricsreinforcedconcretebeams” 27B (1996) 235-244 Composites:PartB

[3] KojiTakeda, YoshiyukiMitsui, KiyoshiMurakami, HiromichiSakai, and MoriyasuNakamura“Flexuralbehaviourofreinforcedconcretebeamsstrengthenedwithcarbonfibersheets” Composites Part A27A (1996) 981-987

[4] G. Spadea, F. Bencardino, and R. N. Swamy, “Structural Behavior of Composite RCBeams with Externally Bonded CFRP,” Journal of Composites for Construction, Vol. 2, No. 3, August 1998, pages 132-137.

[5] “Contribution of externally bonded FRP to shear capacity of RC flexural members,” by Ahmed Khalifa, William J. Gold, Antonio Nanni, and Abdel Aziz M.I. 195-202 in Journal of Composites for Construction, Vol. 2, No. 4, November 1998.

[6] N. F. Grace, G. A. Sayed, A. K. Soliman, and K. R. Saleh, “Strengthening Reinforced Concrete Beams Using Fiber Reinforced Polymer (FRP) Laminates,” ACI Structural Journal, September-October 1999, 865-875.

[7] B. Taljsten and L. Elfgren, “Evaluation of various application techniques for strengthening concrete beams for shear using CFRP-materials” 87–96 in Composites: Part B 31 (2000)

[8] Antonio Nanni, Ahmed Khalifa, Ahmed Khalifa, Ahmed Khalifa, Ahmed Khalifa, Ahmed Cement&ConcreteComposites22(2000)165-174 "Improving shear capacity of current RC T-sectionbeams using CFRPcomposites"

[9] Thanasis C. Triantafillou and Costas P. Antonopoulos, "Design of concrete flexuralmembers strengthened in shear with FRP," Journal of Composites for Construction, Vol. 4, No. 4, November 2000, pp. 198-205.

[10] V.P.V. Ramana, T. Kant, S.E. Morton, P.K. Dutta, A. Mukherjee, and Y.M. Desai "BehaviorofCFRPCstrengthenedreinforcedconcretebeamswithvariousdegreesofstrengthening" Composites:PartB31(2000)461-470

[11] D. Kachlakev and D.D. McCurry, “Behavior of full-scale reinforced concrete beams retrofitted for shear and flexural with FRP laminates,” 445-452 (2000)445-452 (2000)445-452 (2000)445-452 (2000)445-452 (2000)4

[12] Alex Li, Cheikhna Diagana, and Yves Delmas, “CRFP contribution to shear capacity of reinforcedRCbeams,” EngineeringStructures23(2001)1212–1220.

[13] J. F. Bonacci and M. Maalej, “Behavioral trends of RC beams reinforced with externally bonded FRP,” Journal of Composites for Construction, Vol. 5, No. 2, May 2001, 102-113.

[14] Zahra Riahi and Farzad Faridafshin, "Seismic retrofit of reinforced concrete bridges using fibre reinforced polymer composites: state-of-the-art review," in Proceedings of Earthquake Engineering, pp. 1-12, Beijing, China, October 2008.

[15] Swamy Jones and Bloxham, "Structural behaviour of reinforced concrete beams enhanced by epoxy-bonded steelplates," Journal of Structural Engineering, Vol. 65, No. 2, pp. 59–68, 1987.