

Passive Control Reinforced Concrete Frame Mechanism with High Strength Reinforcements and Its Potential Benefits Against Earthquakes

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Abstract:

Severe earthquakes continue to cause major catastrophes. Many devices in active, hybrid, and semi-active structural control systems which are used as controllable force devices are costly to build and maintain. The passive control reinforced concrete frame (PCRCF) reinforced with high strength steel only in the columns presented here provides structural systems more resistance to lateral earthquake loadings at comparatively lower cost. The effectiveness is demonstrated by a nonlinear static analysis using fiber model for a single story single bay frame. The study shows that the use of high performance steel in columns prevents formation of plastic hinges at the critical column base sections and failures are always initiated by reinforcement yielding at the beam ends. Furthermore, after experiencing severe lateral drift, the passive control design has small residual displacements compared to ordinary reinforced concrete frames. PCRCF rehabilitation and strengthening can be achieved more easily as compared with ordinary reinforced concrete frame.

Keywords: earthquake; passive control; high strength reinforcement; failure mechanism; residual displacement

Introduction

In most reinforced concrete (RC) structures, a large stiffness is needed in order to limit structural deformation for service load conditions. In seismic resistant structures, however, the energy dissipation demands are imposed and inelastic deformations are permitted in special detailed regions of structures when the severe earthquake attacks. In particular, moment resistant frames designed according to the strong column/weak beam concept are expected to undergo inelastic deformations by forming plastic hinges in the beams. The columns are supposed to remain elastic to maintain vertical load carrying capacity and prevent possible collapse. Although the required flexural strength difference between beams and columns at joint locations enforces this ideal frame deformation mechanism, the deformations at the base of the first story columns must be excessive to initiate the frame to sway^[1]. Therefore, the formation of plastic hinges at the base of the first story columns is inevitable as shown in Fig.

1. Although in some instances, the formation of plastic hinges at the column bases may not be so critical regarding the safety of the structure, these formation requires extensive rehabilitation efforts.

Moreover, the frame does not possess the recentering ability after undergoing severe lateral drift during strong shaking, and the chances of complete demolition of the structure are always there in case of excessive yielding at the column base sections. Furthermore, the possibility of exceeding the moment capacity at the top of the first columns

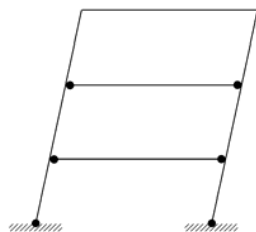


Fig. 1 Strong column/weak beam configuration

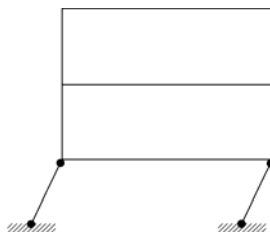


Fig. 2 Soft first story failure

This paper is to describe the alleviation and prevention of the formation of plastic hinges in frame columns by introducing high strength steel reinforcement in RC frame columns, which is called here after as passive control RC frame (PCRCF). (Vasanthy and Jeganathan 2007, Vasanthy et.al., 2008, Raajasubramanian et.al., 2011, Jeganathan et.al., 2012, 2014, Sridhar et.al., 2012, Gunaselvi et.al., 2014, Premalatha et.al., 2015, Seshadri et.al., 2015, Shakila et.al., 2015, Ashok et.al., 2016, Satheesh Kumar et.al., 2016).

1 Mechanism of PCRCF

A conventional designed moment resistant frame usually cannot successfully develop its ability against unexpected earthquake loadings due to limited flexural strength and the formation of plastic hinges at the base of the first story columns. Excessive yielding at the column base sections may lead to eventual collapse, and the soft first story failure mechanism is difficult to avoid. Moreover, even after the survival of structure against extreme lateral drift, the large residual deformations may suggest the need for complete demolition. By introducing high strength reinforcement in columns, PCRCF can safeguard its column base section from excessive yielding and can resultantly adjust structural characteristics by using the reserve flexural strength at the column base sections. Furthermore, the yielding will only occur at beams ends. Due to elasticity of high strength reinforcement in columns, recentering capacity can be improved with the reduced residual lateral displacement under extreme lateral loading. Therefore, repairs can be made easier. (Manikandan et.al., 2016, Sethuraman et.al., 2016, Senthil Thambi et.al., 2016, Ashok et.al., 2018, Senthilkumar et.al., 2018,).

2 Analysis Models and Method

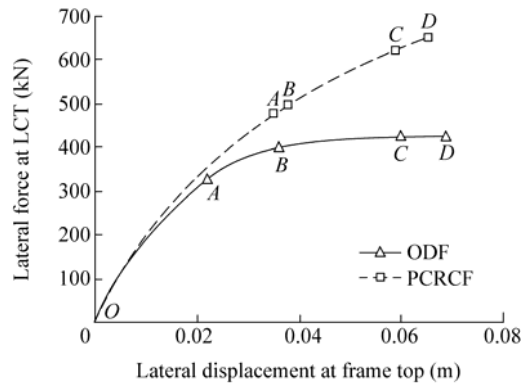
To demonstrate the PCRCF mechanism and to investigate the behavior difference between the ordinary frame and the PCRCF, two single-story single-bay frames, ordinary frame (ODF), and PCRCF, were analyzed. The behaviors and failure mechanism of both the frames were estimated with nonlinear static analysis. Figure 4 presents the selected geometry and the loading pattern for both the frames.

3 Analysis of Results

Response stages

For comparative study, both the ODF and PCRCF were analyzed and the results at each of the response stages were described. The lateral load and displacement relations are shown in Fig. 9 for

the ODF and PCRCF. The lateral load displacement relation of both the frames can be divided into four response stages. The end of each response stage is marked as *A*, *B*, *C*, and *D*. The four response stages are described as follows.



FAILURE MECHANISM

From the observed fiber strains which are summarized in Table 4, the extent of yielding at the four response stages at the critical sections can easily be studied. Moreover, the strains also provide guidance in the exact determination of the failure mechanism in each of the frame studied. From the available data in Table 4, Fig. 10 shows the location of plastic hinges in the frames at the 4 response stages studied.

Figure 10 and values of the fiber strains given in Table 4 show that at the response Stage 2, the failure mechanism developed in the ODF; however, PCRCF yielded only at BRE. Further even up to Stage 4, potential failure mechanism did not appear in the PCRCF. After the first significant yield at RCB, the ODF has shown displacement ductility of smaller magnitude as compared to the PCRCF. The lateral displacement values given in Table 3 show that the ODF at the end of response Stage 1 has 22.0 mm lateral displacement and at response Stage 2 ended was 36.0 mm, where failure mechanism developed in the ODF. However, the PCRCF laterally displaced to 35.0 mm at the end of response Stage 1 and until the end of the

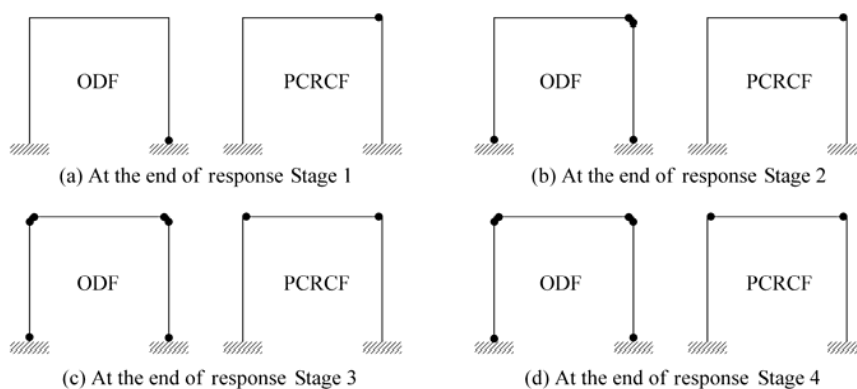


Fig. 10 Location of the plastic hinges in ODF and PCRCF

4 CONCLUSIONS

The passive control RC frame with high strength reinforcement and its expected benefits against earthquakes has been compared with ordinary RC frames. Two single-bay single-story frames were selected and compared. The following conclusions can be drawn.

(1) The PCRCF prevents the soft story failure and provides more lateral load resistance capacity with less repairable cost by simple replacement of ordinary conventional steel in the frame columns with high tensile strength steel.

(2) The PCRCF shows signs of distress mainly at the beam end sections which are potentially safe from the stability point of view of the entire frame as compared with the ODF where column base sections are badly yielded.

(3) Compared to the ODF, the PCRCF rehabilitation and strengthening is easier because the repairs focus on beam end sections instead of the more restricted column base sections.

(4) PCRCF reduces the residual displacement in the frames after the large lateral displacement.

(5) The PCRCF mechanism reduces the chances of complete demolition by avoiding excessive yielding at column base sections.

The performance of PCRCF can be further improved by providing concrete confinement at the beam ends and column base sections, since confinements at the beam and column ends, as well as high strength steel reinforced columns, increase the ultimate deformation capacity at the plastic hinges, and raise the deformation capacity of the whole frame. Since the demonstration of the PCRCF mechanism has been performed by using the single-story single-bay frame, the PCRCF response needs to be demonstrated for multi-story frames with dynamic loadings in future studies. It may be helpful to mix

some proportion of the high performance steel with ordinary one to achieve the response benefits. Hence, the optimum use of high performance steel in multi-story frame columns also needs to be investigated.

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